

TECHNICAL REFERENCE



Galataport Project

Istanbul, Turkey



Diaphragm Wall

Jet Grouting

Deep Soil Mixing

Stone Columns

**Ground Anchors
& Micropiles**

Owner :

Galataport İstanbul Liman İşletmeciliği ve Yatırımları A.Ş.

Main Contractor :

TREVI Spa

Duration of works:

2016 - 2019

Introduction

After a long period of inactivity in Turkey, Trevi got back into action by taking part in one of the most prestigious projects in Istanbul of the last decade: **the Galataport Project, located in the ancient district of Galata, now Beyoğlu, in the heart of the historical city center.**

As one of the largest waterfront projects on earth, Galataport Istanbul has a total investment value of \$1.7 billion including the tender bid, covers a length of 1.2 kilometers of coastline on the Bosphorus and a total area of 100,000 m². This unique coastline, which has been closed to public access for approximately 200 years, will be opened for the use of Istanbulites and the visitors of the city.

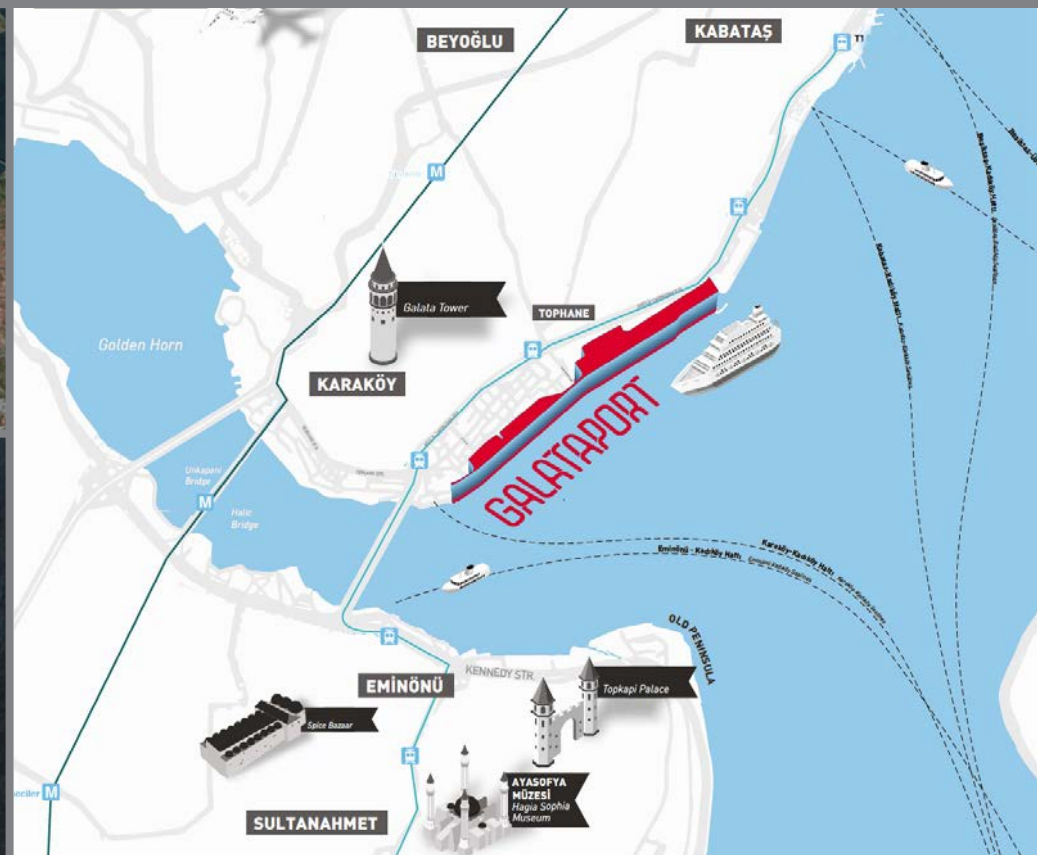
Buildings, required the use of a wide range of technologies for the execution of retaining structures, deep foundations and soil improvement in a high seismicity context.

The technical solutions adopted required the use of equipment prepared to get the highest performance, like the new hydromill Soilmec SC-135 and the new rig Soilmec SR-95 in TurboJet version.

The project

The Project is divided in two packages: Salıpaزاری and Karaköy, which were firstly issued as two different tenders and were eventually both awarded to Trevi.

The concept tender design was deeply studied by the engi-



Designed to be quite unlike anything else, the terminal is being built underground so that domestic and foreign tourists can continue their journey without any interruption. The plans of the project include roughly 250 food retail points over a total surface of 52,000 m² and office spaces over 43,000 m². The 177-room Peninsula Istanbul will make use of three protected buildings and the hotel lobby will be built inside the Karaköy Passenger Hall, Turkey's first modern cruise terminal, which dates back to the 1940s.

Hosting the Istanbul Modern Museum and Istanbul Painting and Sculpture Museum of Mimar Sinan Fine Arts University, this area will mark the first museum square of Istanbul.

Galataport Istanbul is projecting to welcome 25 million visitors, seven million tourists, including 1.5 million cruise passengers and crew per year. The complexity of the project, which included the construction of underground structures both in open areas and underneath the existing historical bu-

neers of Trevi Design, Research & Development office, who proposed several technical improvements and alternative solutions, which resulted in a more sustainable design strategy and a cost saving to the client. Therefore, Trevi acted as both main contractor and proposal designer for all geotechnical engineering works.

The difficult hydraulic and geotechnical condition, the seismic hazard of the site, the presence of historical buildings that need to be strengthened, the simultaneous reconstruction of the quay by other Contractors and the continuative operation of the port while running of construction activities, required the employment of cutting-edge technologies within the foundation engineering field.

The Salıpaزاری package included the construction of an underground cruise port terminal, for which the whole 65,000 m² large site needed to be excavated down to ap-



Deep Soil Mixing (by Turbojet®):
250,000 m³, Ø 1500 mm columns to the maximum depth of 28 m.



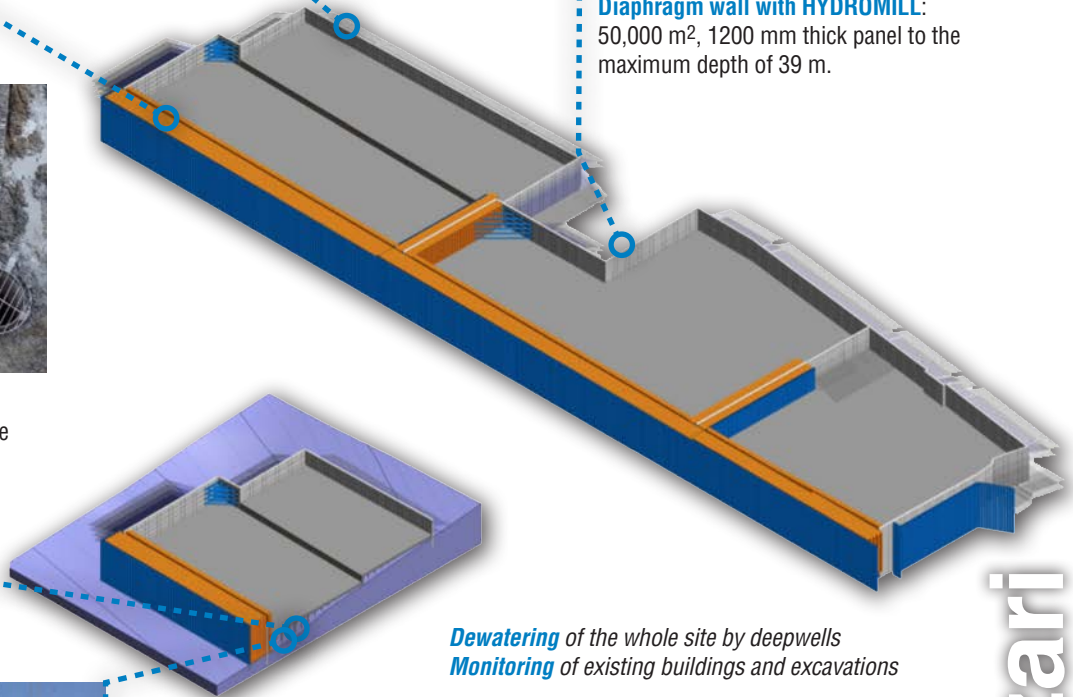
Temporary Ground Anchors:
30,000 m, Ø 165 mm anchors to the maximum depth of 30 m.



Diaphragm wall with HYDROMILL:
50,000 m², 1200 mm thick panel to the maximum depth of 39 m.



Bored Tension Pile:
35,000 m, Ø 1000 mm piles to the maximum depth of 15 m.



Dewatering of the whole site by deepwells
Monitoring of existing buildings and excavations



Vibro replacement:
100,000 m, Ø 800 mm columns to the maximum depth of 15 m.



Kılıç Ali Paşa
Mosque

Tophane
Fountain

Nusretiye
Tower

Nusretiye
Mosque

Mimar Sinan
Museum

Istanbul Modern
Museum

Cruise terminal

Salipazari

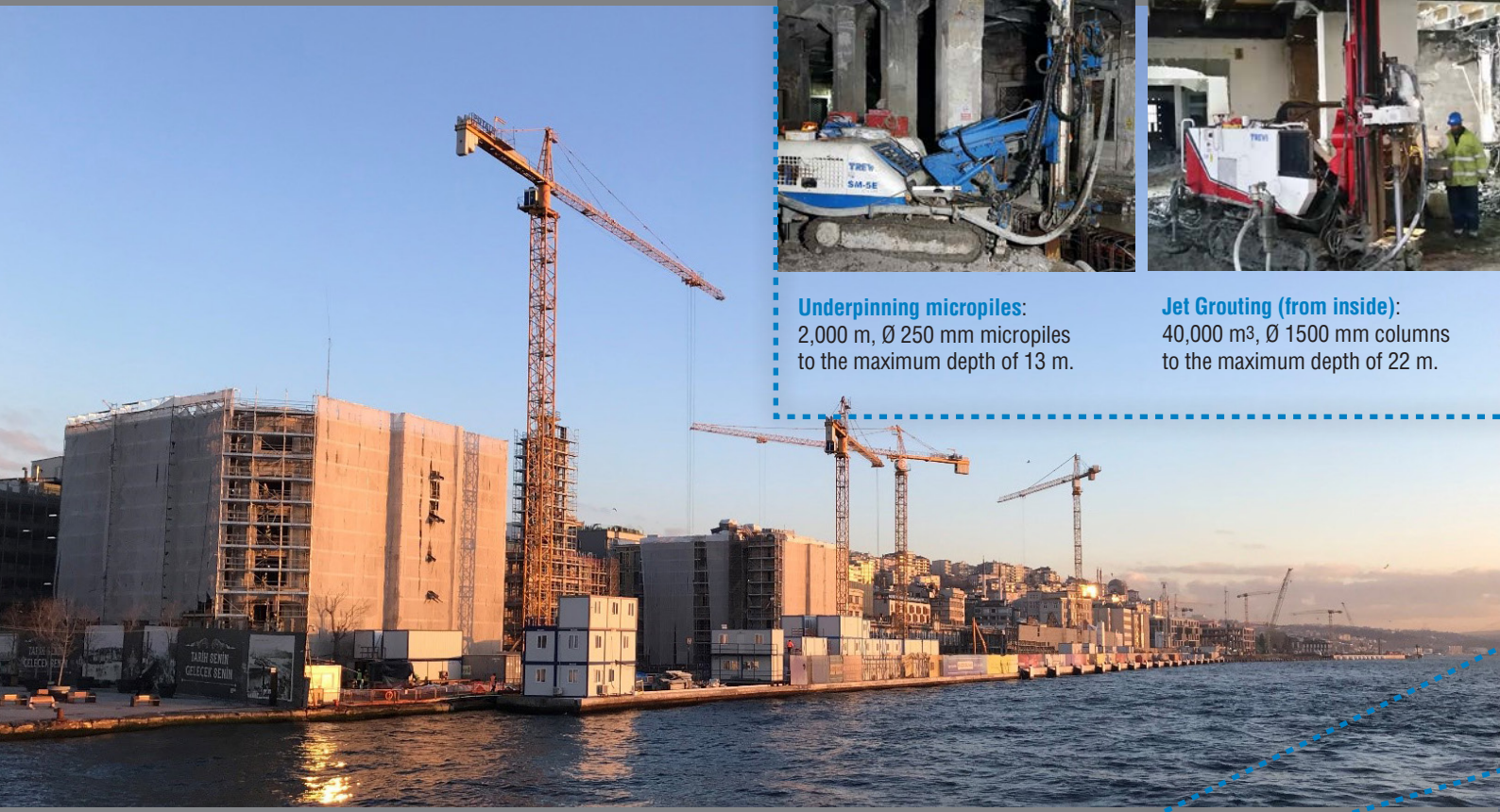
proximately 15 m below ground level (13 m below sea level). The ground surface on top of terminal has been designated for seaside promenade, walking paths and public squares. The related foundation works included the execution of a 1,200 mm thick anchored diaphragm wall, which was executed by both mechanical grab and hydromill, (50,000 m² of D-Wall and 30,000 m of ground anchors), a massive soil improvement against liquefaction by vibro replacement (100,000 m of stone columns) and Deep Soil Mixing by TurboJet® (250,000 m³ of treated soil), a pile foundation composed by 1,000 mm diameter bored piles (35,000 m) and the implementation of a dewatering system for the whole site.

The **Karaköy package**, in addition to the re-construction of its portion of quay, consisted of the refurbishment of five exi-

single fluid jet grouting (40,000 m³ of treated soil) and structural underpinning by means of micropiles (2,000 m), in order to allow the excavation of a partial basement underneath the existing foundations.

For both Salıpazarı and Karaköy packages, Trevi installed and managed a massive monitoring system to keep under control the movement and forces acting on the different components of foundation system, the surrounding soil and existing buildings.

All above listed activities were be executed almost simultaneously, which brought additional difficulties to the project.



Underpinning micropiles:
2,000 m, Ø 250 mm micropiles to the maximum depth of 13 m.



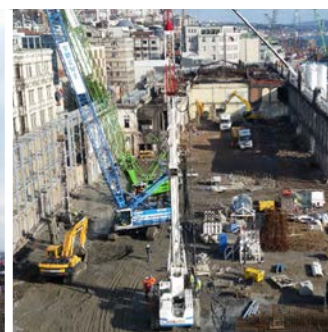
Jet Grouting (from inside):
40,000 m³, Ø 1500 mm columns to the maximum depth of 22 m.

isting buildings. The non-historical buildings were demolished and the foundation works within their footprint consisted of a 1,000 mm thick propped diaphragm wall/secant pile wall, which were executed by both mechanical grab (18,000 m² of D-Wall with 200 steel props and 7,000 m of secant piles), an hydraulic bottom plug in order to reduce groundwater flow during excavation and related settlements of adjacent buildings, executed by 2,000 mm double-fluid jet grouting overlapping columns (45,000 m³ of treated soil), a lattice-type soil improvement against liquefaction composed by elliptical jet grouting columns (50,000 m³ of treated soil) and a mesh of permanent tension micropiles against uplift, composed by Double Corrosion Protected (DCP) steel bars (10,000 m).

For what concerns the hystorical buildings that were preserved, the related foundation works needed to be conducted from inside the buildings, with reduced headroom and consisted of a massive soil improvement against liquefaction by



Jet Grouting bottom plug:
45,000 m³, Ø 2000 mm columns to the max depth of 29 m.



Jet Grouting Elliptical:
50,000 m³, 4000 mm long columns to the max. depth of 25 m.



Mass excavation (100,000 m³), **Steel strutting** (200 struts, 1500 t) and **Dewatering** by deep wells.

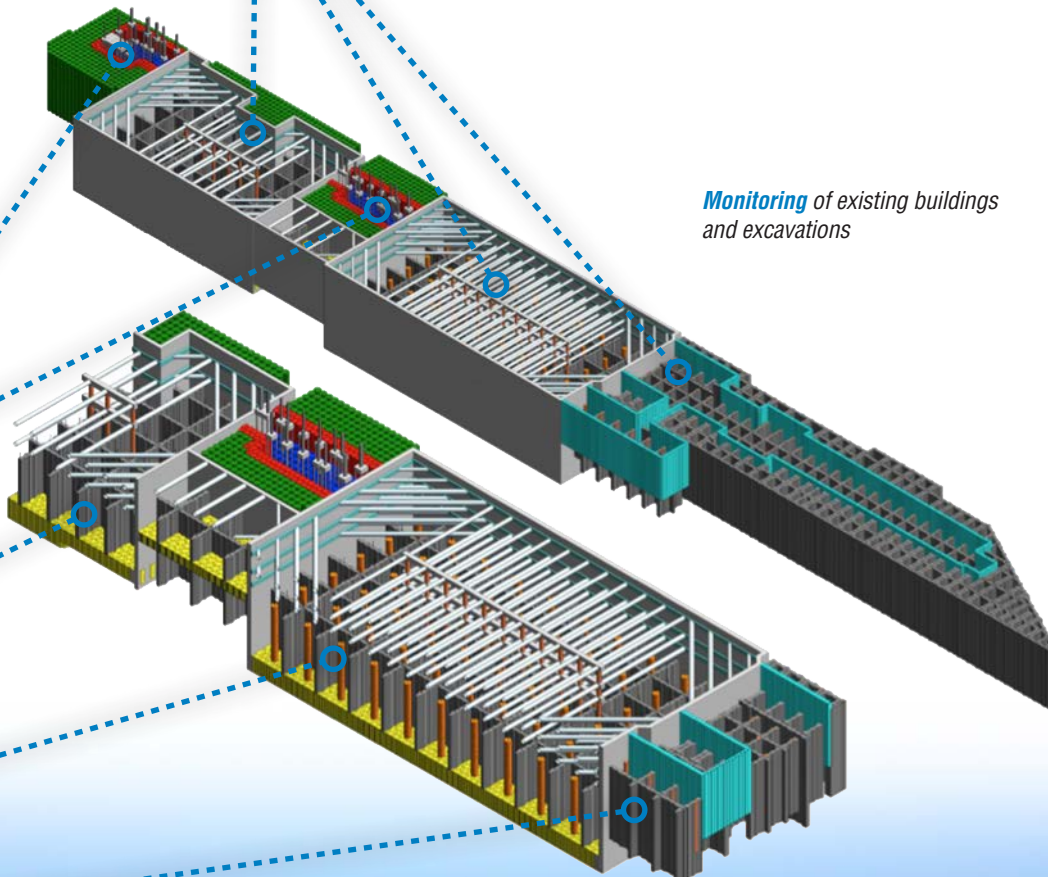


Diaphragm wall (Mechanical Grab): 18,000 m², 1000 mm thick panels to the maximum depth of 35 m.



Secant piles: 7,000 m², Ø 1000 mm piles to the maximum depth of 30 m.

Bored tension piles: 3,000 m, Ø 1000 mm piles to the maximum depth of 30 m.



Monitoring of existing buildings and excavations

Karaköy



Micropiles (permanent): 10,000 m, ø 250 mm micropiles to the maximum depth of 26 m.

Geology

The soil encountered along the seaside alignment of the diaphragm wall consists of about 15 m to 20 m of imported, non-compacted, gravel/cobble size fill, overlying an approximately 5 m to 10 m thick layer of very loose ($NSPT=0-15$, $qc=0.5-3.0$ MPa), liquefiable silty sand, containing an exceptionally high amount of shells, concentrated in the top 3 to 6 m. The bedrock is very strong ($UCS=100-150$ MPa), very fractured Greywacke; it is found at a depth ranging between 25 m to 45 m below sea level and it is covered by an approximately 2 m thick layer of stiff clay, with a $c_u=200-300$ kPa.

UNCONTROLLED FILL



VERY LOOSE SILTY SAND



VERY STIFF CLAY



VERY WEAK TO MODERATELY STRONG GREYWACKE



Focus on Trevi intervention: The new quay of Salpazari

An interesting design solution was adopted for the seaside section of the new quay wall and terminal building. The new quay is composed of a combi-wall, formed by bored-cast-in-situ piles, excavated through a driven permanent steel casing, which was installed from a barge, while demolition of the existing deck-on piles was started (**Figure a**).

The main alignment of contiguous piles is supported by inclined

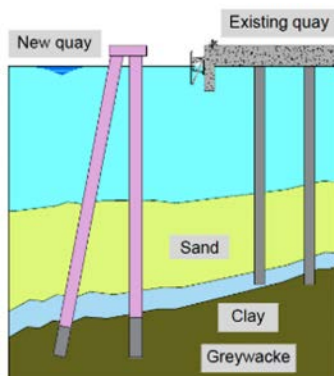
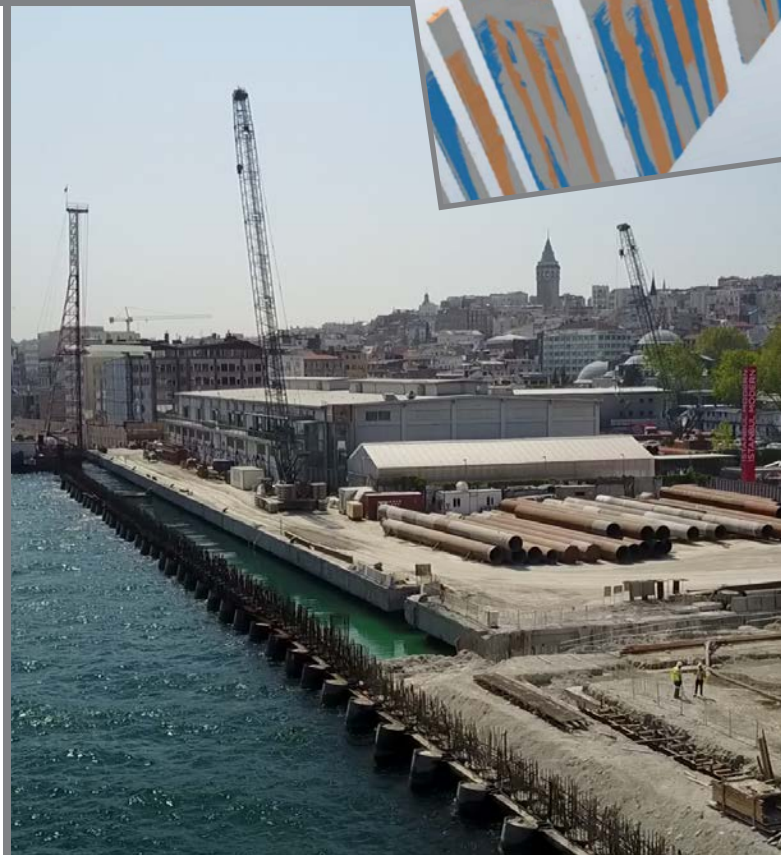
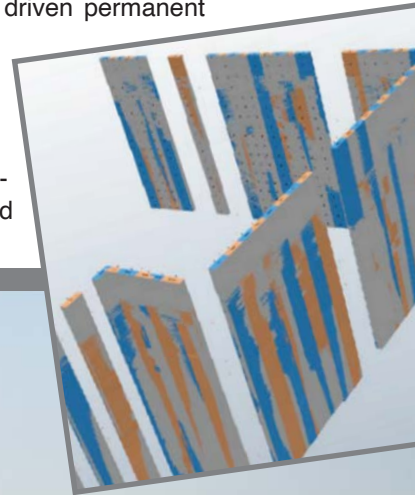


Figure A

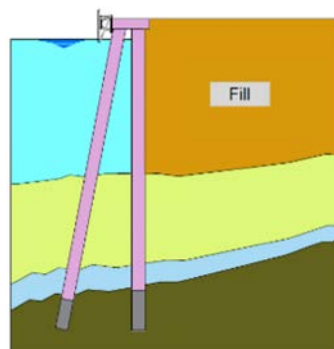


Figure B

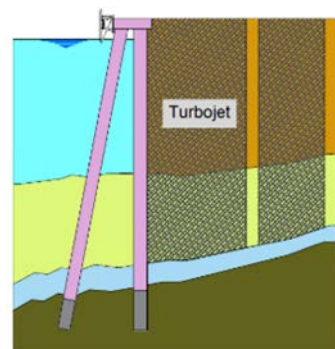


Figure C

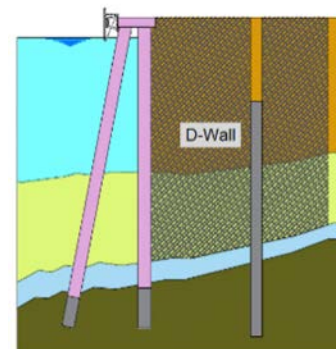


Figure D

piles installed at the center/center distance of 5 m. Subsequently, the area was backfilled with a gravel-size controlled fill (**Figure b**), which was lately improved by a massive deep soil mixing treatment, consisting of 1500 mm diameter tangent columns (**Figure c**).

The soil improvement had three main goals:

- to stabilize the loose man-made fill prior to the execution of the diaphragm wall,
- to reduce the risk of liquefaction of the in-situ loose sand below natural seabed level,
- to create a stiff block between the new quay wall and the diaphragm wall/terminal building, in order to properly dissipate the ship load.

Deep soil mixing columns were executed on a triangular pattern and an empty row was left in correspondence of the future diaphragm wall alignment. At this stage, the 1200 mm thick diaphragm wall was constructed (**Figure d**). The diaphragm wall has a reduced cut-off level and the area between new quay and diaphragm wall is designed to be a gangway platform for passengers.

The Deep Soil Mixing treatment was executed down to the stiff clay layer (*maximum depth of approximately 25 m below the working platform*), while the diaphragm wall toe level was at 32 m. Once the diaphragm wall was completed, the bulk excavation and construction of gangway started, so that the new quay and the diaphragm wall were permanently connected (**Figure e**). Activities were finalized by completing the

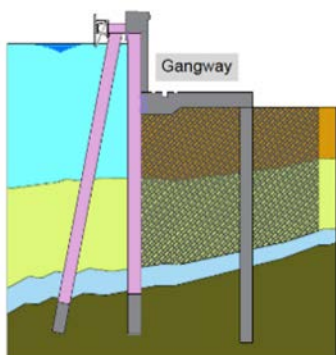


Figure E

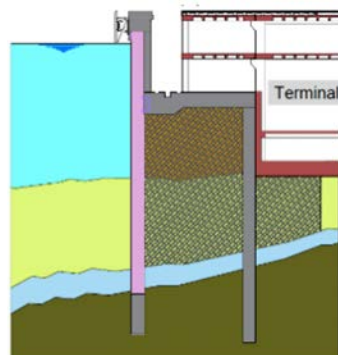


Figure F

whole excavation down to 13m below sea level, constructing the terminal building and removing the inclined support piles at seaside (**Figure f**).

The two Soilmec SR-95 rigs were equipped with the Soilmec automatic Drilling Mate System (DMS), specifically developed for construction equipment, which allows operators to monitor and manage the machine.

The system is capable of automatically driving the rig during drilling and lifting phases, while grouting is being carried out.

The DMS was also mounted on the hydromill Soilmec SC-135 Tiger, which allowed to generate a virtual 3D reconstruction of the executed D-Wall panels, based on the data recorded in the execution of diaphragm walls, in order to detect any possible problems or undesired deviation.

The soil improvement underneath existing buildings in Karaköy

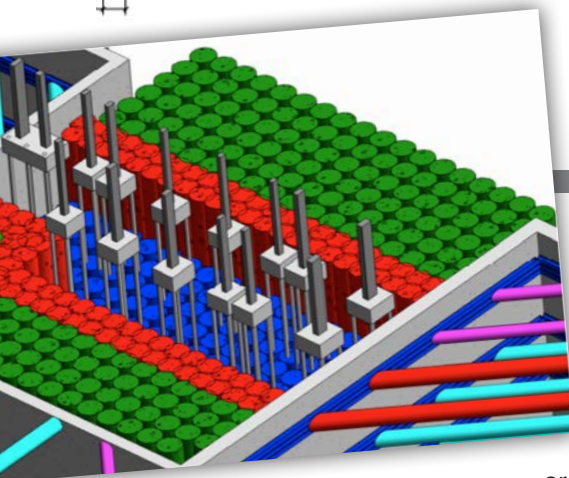
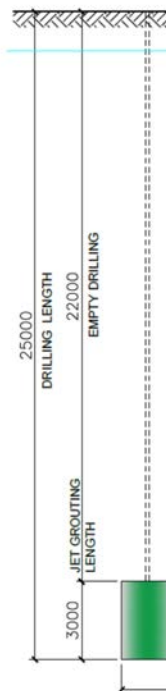
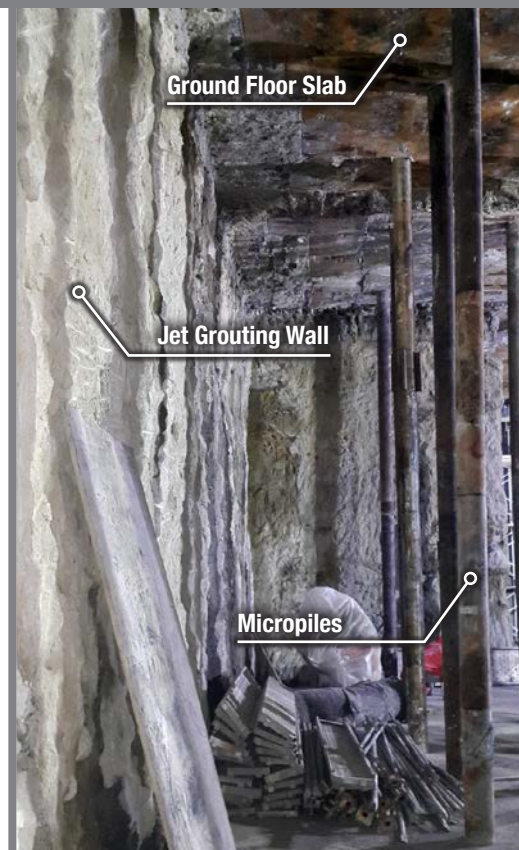
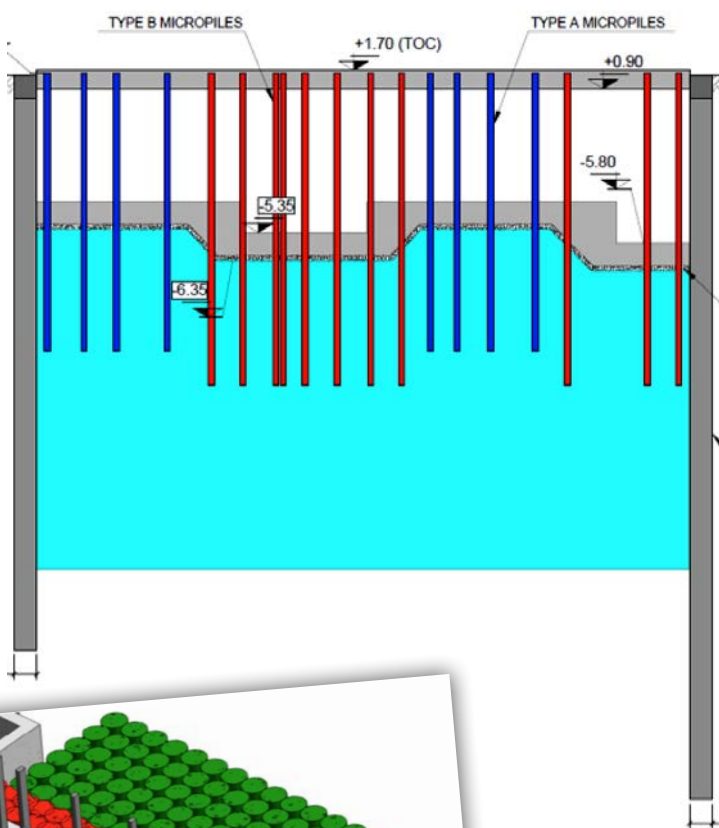
Some historical buildings in Karaköy were registered as first-degree cultural heritage and needed to be completely preserved during and after construction works. They were restored to be part of the future port facilities. Since the existing structures and foundations had to be strengthened, the mechanical properties of the bearing soil needed to be improved in order to sustain the new increased loads.

For this reason, 1.5 m diameter, tangent single-fluid jet grouting columns were designed by the Engineer, which had also the scope of reducing the risk of liquefaction of the loose in-situ fill and sand. Since a partial basement (excavation down

Therefore, micropile maximum length was 13.5m. All activities were carried out from inside the existing buildings, with a reduced headroom (*maximum free height of 3 m*). All jet grouting columns and micropiles were executed with Soilmec mini-drilling rigs (*SM-103 and SM-5*), both with electric engine.

The soil improvement for the new buildings in Karaköy (the multi-purpose bottom plug)

Among all interesting geotechnical works executed in Karaköy site, a special mention needs to be made to the Multi-Purpose Bottom Plug (MPBP). This innovative geotechnical was employed for the excavation within the footprint of the



to - 5.2 / - 6.5m msl) needed to be excavated underneath the existing buildings, the jet grouting columns were overlapped

around the area of excavation and worked as a watertight gravity retaining wall during the excavation. In order to allow the excavation of the partial basement, a set of underpinning micropiles was designed by the Engineer. All jet grouting columns were executed to the maximum depth of 21.5 m (*from WPL at +1.5 msl to toe level at -20.0 msl*), while micropiles were executed from the working platform level of +1.6 m msl and toe level ranged between -10.5 m and -12.0m msl.

demolished buildings, to allow their reconstruction with 2 or 3 basement floors.

The MPBP solution is a combination of seepage control, ground improvement, and load bearing elements, which was adopted to allow the deep excavation within the footprint of demolished buildings. Excavation depth was approximately 12.5 m below the working platform (*11.0 m below sea level*).

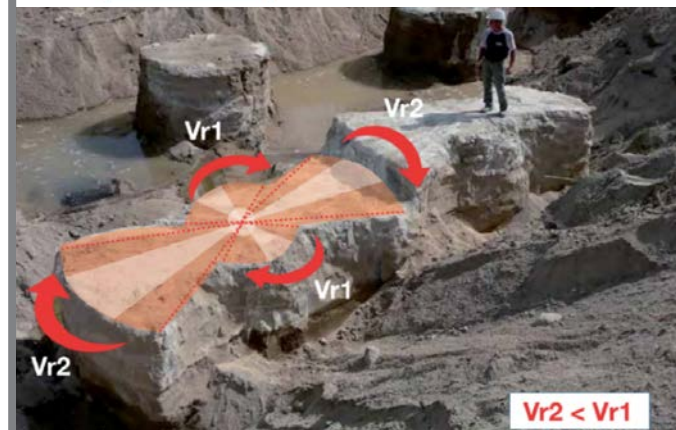
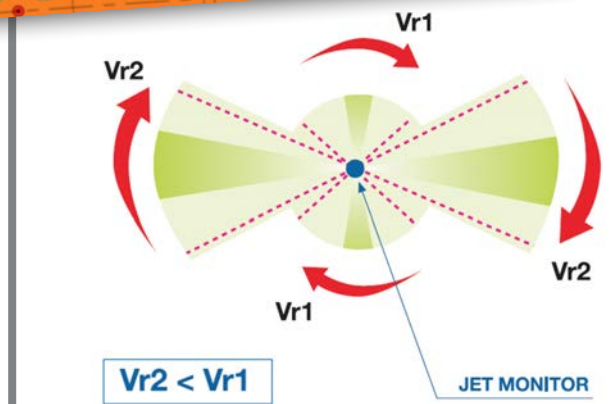
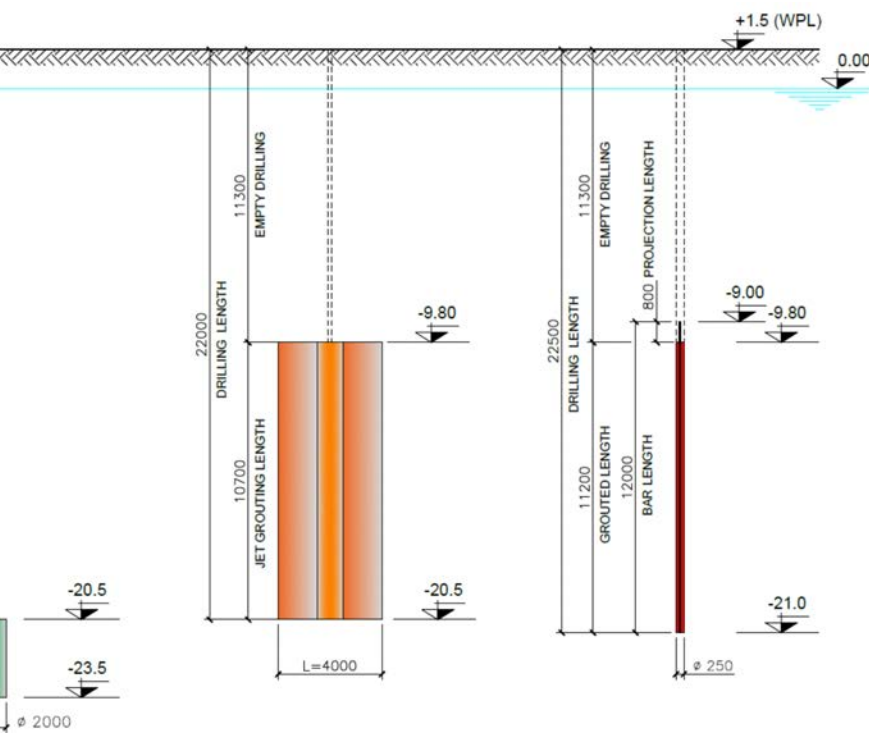
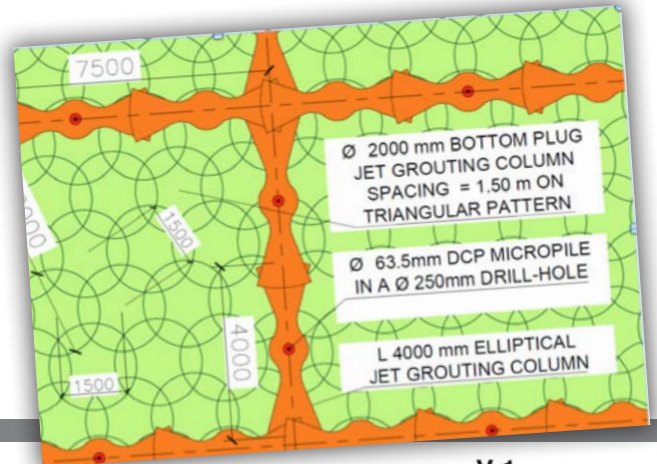
The main body of the bottom plug is composed of overlapping jet grouting circular columns, executed with the double-fluid method and designed to have a 2,000 mm diameter. Columns are executed on a triangular pattern with an average centre to centre spacing of 1,500 mm.

The bottom plug's thickness is the one strictly necessary to ensure the hydraulic stability of the excavation. Subsequently, pseudo-elliptical jet grouting columns are executed from the top level of the bottom plug up to the excava-

tion level on a lattice-type pattern, in order to act as a mitigation element to liquefaction. In particular, the pseudo-elliptical shape of jet grouting columns was chosen instead of the circular one in order to optimize the lattice geometry, by minimizing the amount of jetting and related time of execution. The lattice-type jet grouting treatment also acts as a foundation for the future building and will carry the vertical structural loads in permanent conditions. Average width in plan of elliptical jet grouting column is 4.0 m.

Finally, the stability of building against uplift is ensured by permanent Double Corrosion Protected (DCP) steel bars, which are installed as drilled and grouted micropiles through the previously-executed elliptical jet grouting columns. DCP steel bars have a diameter of 63.5 mm and are installed within a

ting phase in a planned way. The rotation speed varies from Vr1 to Vr2 to create variable radius sectors.

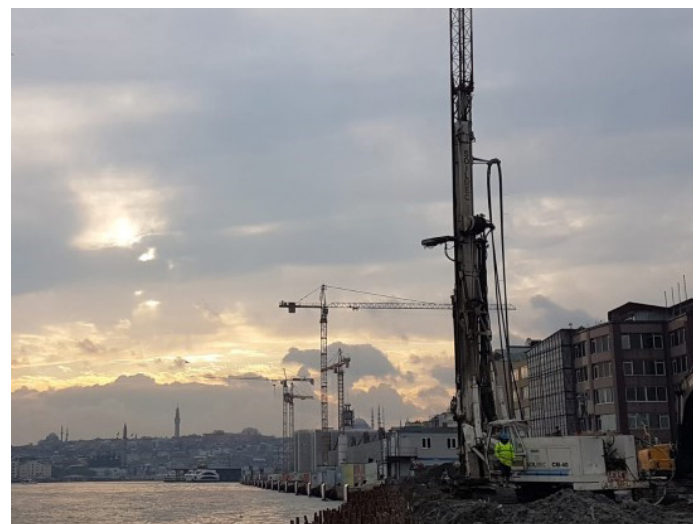


250 mm diameter drill-hole.

The technologies involved and the electronic control devices employed for the execution of MPBP are of latest-generation. In particular, for the execution of the double fluid jet grouting bottom plug, the drilling rigs were equipped with the "Jet-Vision System" and with the "Drilling Position System" (DPS) electronic devices.

The former controls, monitors and records the automatic rising of the drilling tool/rods during the jetting phase; the latter monitors and records the 3D position of the drilling tool during the drilling phase, which is of basic importance to ensure the overlapping between the jet grouting columns and the consequent successful execution of the bottom plug.


For the execution of pseudo-elliptical jet grouting columns, the Jet-Vision system was provided with the "orbital jet" option, which allows to modify the rotation speed during the jet-





**13 different geotechnical
engineering technologies**
in the same project site

650 diaphragm wall panels for
a total surface of **68,000 m²**
(10 soccer fields)
with **80,000 m³** of concrete
(32 Olympic swimming pools)
and **6,000 t** of steel
(almost 1 Eiffel tower)



The new landmark of the Bosphorus shore
with **100,000 m² of total surface**
and **1.2 km of coastline**



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3 cruise ships simultaneously,
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